2024 PLS-CADD Advanced Training and User Group

Developing Security Loads

Jean-Pierre Marais

by

Research Credit : Significant contributions by David Folk



IT'S ALL ABOUT YOUR POWER LINES



IT'S THE SOLUTION

Session Overview

- 1. Should we care about security loads?
- 2. Quantifying and modelling broken wire loads
- 3. Avoiding longitudinal cascades

Nature & frequency of broken wire vs failure cascades

Dx vs. Tx considerations

Early and recent research findings

Modelling in PLS CADD

Key design features to avoid longitudinal failures

Modelling in PLS CADD

Case study

Should we care about security loads?



Longitudinal Load Sources & frequency

Longitudinal Cascades

Broken Wires





On average, EPRI is notified of a cascade every second year



On average, EPRI is aware of multiple broken wire events every year

Failure Containment vs. Broken Wire Events

Cascading Event





https://www.youtube.com/shorts/u2ES5TdT_Vo

Failure Containment vs. Broken Wire Events



Security Load Design



Broken Wire Loads



Early research into broken wires

principal investigator A. H. PEYROT University of Wisconsin project manager M. SILVA Electric Power Research Inst



EPRI's Dynamic Impact Test Line -Research Objectives

- Develop confidence in broken wire load prediction though full-scale test results
- Include the impact of true conductor rupture
- Verify and calibrate accuracy of FEM models simulating broken wire
- Develop a simple, easy to use, empirically based formula for calculation of broken wire events



Leading Parameters



Dynamic Impact Test Line Schematic

Critical Broken wire Scenario: Rupture one span away from strain structure



Variable from 860 -9350lb/in

Variable from 13-29%UTS

 \sim

Deadend Support

Impact of conductor rupture vs. quick release

- Previous broken wire tests used quick release to simulated broken wires
- Wire rupture has the potential to release additional axial shock load into conductor
- Wire rupture in tests achieved by 2 methods;
 - Rebar cutter light conductors
 - Isolating king wire and allowing EDT to rupture it
- Conductor rupture compared to quick release results
 - Moderate increase (9%) in impact load at strain pole
 - No impact on suspension pole loads





Broken Wire Test Time History



10.0



Ebbi

Test 28 750ft Span 1680 Chukar ACSR @ 21%RBS, 10776lbs Insulator Length = 11.33ft Susp DLF = 1.25 | Strain DLF = 1.15

0	

8





Test 28 750ft Span 1680 Chukar ACSR @ 21%RBS, 10776lbs Insulator Length = 11 33ft Susp DLF = 1.25 | Strain DLF = 1.15

Preliminary Results



Preliminary Results vs Historical Results (full scale and model tests)



Calibrated Finite Element Modeling

- FEM is being completed in parallel with full scale testing
- Full scale testing is being used to calibrate an FEM model
- FEM will allow for extrapolation of line parameters beyond what is feasible at the Lenox site
- Initial results show strong correlation between FEM peak loads and experimental results
- Time histories of the FEM also show strong correlation with experimental results



Calibration of FEM model essential for accurate FEM predictions

6 _{fem}	19	19 _{FEM}	20	20 _{FEM}
.28	0.95	0.91	1.87	1.88
.18	0.97	0.90	2.02	1.92

Significant findings to date

- Strong correlation between Normalized Weight Span Ratio and Dynamic Load Factor
- Small ratio between strain and suspension tower loads (1.010)
- Weak correlation between insulator length and DLF
- Varied (indeterminate) correlation between structure stiffness and DLF

Ongoing research may produce refinements in empirically based DLF prediction

L, w & H are the most significant variables

Ongoing research to determine impact of Li & k

DLF - Empirically based Calculation

- 795 Drake ACSR Conductor Unit Weight = 1.094 lb/ft
- Span Length = 1000ft
- Tension = 7934.5lb @ 32°F Initial (H/w = 7233.4ft / 2205m)
- Class H3, H-frame, FRP crossarm



Example applied to PLS CADD

• WSR =
$$\sqrt{\frac{1000ft * 1.094\frac{lb}{ft}}{7935lb}} = 0.371$$

- DLF Preliminary Equation = 4.7 * WSR 0.31
- DLF = 4.7 * 0.371 0.31
- DLF = 1.44
- Point Load = Tension * DLF = 7934.5lb * 1.44 = 11388lb

Note: This empirical prediction includes results for a range of insulator lengths and structure stiffnesses

DLF Modeling Method A – L2 Analysis – Adjust Tension %

1. Break line back conductors

2. Apply % Hor. Ten. Command, set target % = DLF

Description	Weather case	Cable condition	Wind Direction	Bisector Wind Dir (deg)	Wire Vert. Load Factor	Wire and Struct. Wind Load Factor	Wire Tension Load Factor
Broken Wire_Adjust Tension %	32 Deg F	Initial FE	NA+	NA	1	1	1

#1		#1	#1	#2	#2	#2
Wire(s)	Command	Value	Wire(s)	Command	Value
			(%)			(%)
Set			(# subcond.)	Set		[£] subcond
Phas	e		(lbs)	Phase		(lbs)
Spar	n		(deg)	Span		(deg)
2:3:Back		Broken Wire (# Broken Subconductors)	10	2:3:Ahead	% Hor. Ten. (changes V, T and L)	144

RESULT

- 1. Warning Message
- 2. Resultant load = 5013lb









DLF Modeling Method B – L2 Analysis – Load Factor Application

- **Clip Insulators** 1.
- Apply wire tension load factor = DLF 2.
- Break line back conductors 3.

Description	Weather case	Cable condition	Wind Direction	Bisector Wind Dir (deg)	Wire Vert. Load Factor	Wire and Struct. Wind Load Factor	Wire Tension Load Factor
Broken Wire_LF	32 Deg F	Initial FE	NA+	NA	1	1	1.44

#1 Wire(s) Set Phase Span	#1 Command	#1 Value (%) (# subcond.) (lbs) (deg)
2:3:Back	Broken Wire (# Broken Subconductors)	10

RESULT

Resultant load on Suspension = 7186lb (includes RSL after insulator swing out)

Section Modify

usabieu

SAPS Finite Element Sag-Tension Options





Clip Insulators (lock unstressed length, force finite element saq-tension)



DLF Modeling Method C – L2 / L3 Analysis – Apply Point Load

- **Clip Insulators** 1.
- Break line back conductors 2.
- Apply Add Long Load Command, set target load = Tension * DLF 3.

Description	Weather case	Cable condition	Wind Directi	d Bisector on Wind Dir (deg)	Wire Vert. Load Factor	Wire and Struct. Wind Load Factor	Wire Tension Load Factor	
Broken Wire_Apply Point Load	32 Deg F	Initial FE	NA+	NA	1	. 1	1	
#1	#1	#1		#2			#	2
Wire(s)	Command	Value	•	Wire(s)			Com	mand
		(%)						
Set		(# subco	nd.)	Set				
Phase		(lbs)		Phase				
Span		(deg)		Span				
2:3:Back+Ahead Broken Wire (# Brok	n Subconductors)		10 2	2:3:Ahead /	dd Long. Lo	ad (wire c	oord. syste	m)

RESULT

- Resultant load = 11388lb 1
- Xarm @ 122.9% 2.
- Pole at 143% 3





SAPS Finite Element Sag-Tension Options

Section Modify

usaple

Clip Insulators (lock unstressed length, force finite element saq-tension)





DLF Modeling Method C – L2 / L3 Analysis – Apply Point Load

~115kV DC Lattice Tower

- Break line back conductors 1.
- Apply Add Long Load Command, set target load = Tension * DLF 2.

Description	Weather case	Cable condition	Wind Direction	Bisector Wind Dir (deg)	Wire Vert. Load Factor	Wire and Struct. Wind Load Factor	Wire Tension Load Factor	Struct. Weight Load Factor	Struct. Wind Area Factor
Broken Wire_Apply Point Load	32 Deg F	Initial RS	NA+	NA	1	1	1	1	1

#1	#1	#1	#2	#2	#2
Wire(s)	Command	Value	Wire(s)	Command	Value
		(%)			(%)
Set		(# subcond.)	Set		[£] subcond
Phase		(Ibs)	Phase		(lbs)
Span		(deg)	Span		(deg)
2:3:Back+Ahead	Broken Wire (# Broken Subconductors)	10	2:3:Ahead	Add Long. Load (wire coord. system)	11388

RESULT

- Resultant load = 11388lb 1
- Max Xarm Member @ 83.4% 2.
- Max Leg Member at 26.2% 3.





PLS CADD modelling summary

- Method A (% Hor. Tension) not appropriate for FE cables
- Method B (Wire Tension LF) will include RSL of suspension insulators (strain structures will be accurate)
- Method C (specify load) Will apply specified dynamic load irrespective of cable model

Cascading Failure Loads



Resiliency and longitudinal cascades

All conductors connected to the structure are affected



Structure Types that are sensitive to cascades

- Structures not designed without specific consideration for longitudinal loads
- Structures that rely on longitudinal support of shieldwire
- Self-supporting structures with reduced longitudinal capacity
- Planar structures are at risk for longitudinal cascades
 - H-frames, especially wood & lattice frames





Consider Impact of Material Type



Wood poles fail suddenly upon rupture

Steel and lattice structures absorb energy

Use of Composite poles in wood pole lines to increase resiliency



- Some utilities are using composite structures to arrest cascades in wood pole lines
 - What loads should the composite pole resist?
 - Can it be too flexible (allowing too much deflection on next wood pole)

Note: Anti – cascade composite pole is a tangent structure with standard post insulators



Equivalent Composite vs. steel pole - cascade resistance





Composite + Wood pole line: Longitudinal Cascade Sequence

Structure fails longitudinally
Loss of tension in all wires of compromised span
Adjacent structure & insulators deflect due to unbalanced load
Intact span loses tension, settles at residual static load (RSL)
Tension imbalance on next wood pole





RSL Modeling – Loads on Composite structure (traditional method)

Model Transmission Line using all M4 models (level 4 cable, clipped in) 1.



Setup Notes					Anti-Cascading Pole Results					
Notes	Test #	Case	Weather	Pole Usage	Insulator Usage	Combined RSL	Тір ∆			
				%	%	lb	ft			
Adjacent structures modeled as M4 models	35	3	30° F 0mph Wind 0" Ice	49.9	91.7	<u>5459</u>	16.79			

Phase Span

Broken Wire (# Broken Subcond

Back Spans

Yes

Weather case Cable Wind Description condition Directio Create Load Case 2. Initial FE NA+ All Wires Broken_30 deg F_comp pole iteration_r0 30 Deg F Structure Loads Criteria Adjust #1 #1 Cable Wire(s) Command Loads Set

n	Bisector Wind Dir (deg)	Wire Vert. Load Factor	Wire and Struct. Wind Load Factor		Wire Tension Load Factor
	NA	1		1	1
		#1 Valu (%) (# subc (lbs (deg	e) ond.) ;)])		
luct	ors)		10		

RSL Modeling - Loads on composite pole and first wood pole (revised method)

- 1. Model Transmission Line using all M4 models (L4 cable , clipped in).
- 2. One-way stringing applied to anti-cascading structure



Section Modify

uisapieu

SAPS Finite Element Sag-Tension Options Clip Insulators (lock unstressed length, force finite element sag-tension)

Dead end - section starts here

- 3. Create Load Case
 - No "Adjust Cable Loads" modifications are required



RSL Modeling - Loads on composite pole and first wood pole (revised method)

- 3. Change Modeling Settings to assume FE analysis
- 4. Conduct Structure Check on anticascading and adjacent structure



Case Study Result: Composite structure not too flexible





CASE STUDY INVOLVING BOTH BROKEN WIRE AND CASCADE





Event Sequence

- Broken wire event on damaged shield wire during wind storm
- Damage from lightning suspected or conductor contact (galloping)
 - Of 7 strands, only 2 were fully intact
 - 2 strands were burned through
 - 3 strands were compromised





Event Sequence

- Excessive deflection resulted in failure of wood pole
- Guy failure on a stay from slippage with corroded internal springs in guy grip locking mechanism







Event Sequence

- Dead end insulator assemblies failed on strain structure
- Cascade continued until next anti-cascade tower





Application of residual static load (RSL) on all phases

RSL = Net longitudinal load following removal of conductor tension in adjacent span, considering both insulator and structure deflection

- Application of RSL is elementary in PLS CADD
 - Provided that, as a minimum -
 - Level 2 cable model is used for rigid (self- supporting lattice) structures
 - Level 3 cable model is used for flexible structures (e.g. steel, wood & composite poles)
 - Additional reductions from purpose designed load reduction techniques may be relevant
- NOTE: Application of RSL to all phases may well not dominate many axisymmetric structures
 - i.e. Overturning from extreme wind > overturning from RSL
- Additional reduction factor for back tension of intact wires may be applicable to suspension / tangent structures (0.8?)

RSL LOADING DID NOT CONTROL THIS 230kV STEEL POLE



Power Line Systems





IT'S ALL ABOUT YOUR POWER LINES

- Feel free to contact me at
 - ***** +1 704 595 2495
 - imarais@epri.com

IT'S THE SOLUTION